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Flood Control Channels Research Program

Modified Laursen Method for Estimating Bed-Material Sediment Load

by Edward B. Madden Consulting Engineer

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Flood Control Channels Research Program

Modified Laursen Method for Estimating **Bed-Material Sediment Load**

by Edward B. Madden **Consulting Engineer** 10109 McCree Road Dallas, TX 75238

Final report

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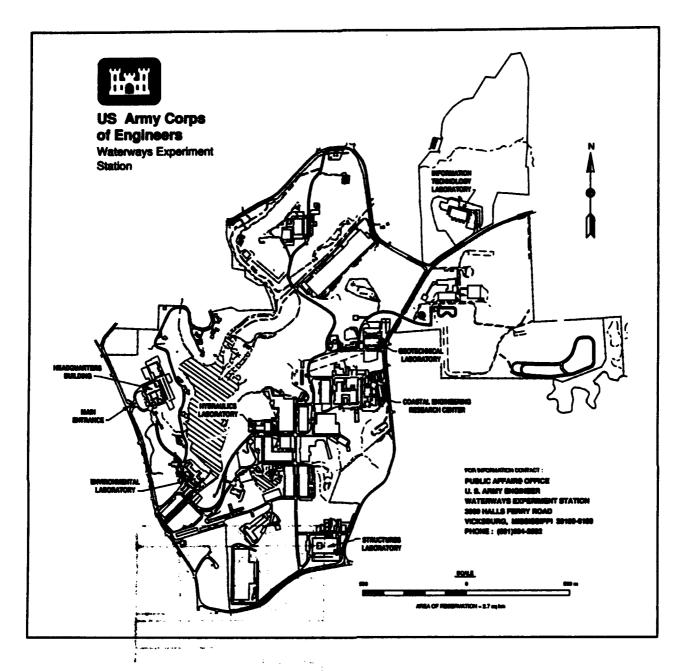
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Preface

The investigation reported herein was conducted for the U.S. Army Engineer Waterways Experiment Station (WES) by Edward B. Madden under Contract DACW39-85-M3699. It documents a modification to coefficients in the Laursen Transport Function using data from streams and rivers. To better fit observations, a new expression involving Froude number of the flow was added to the calculations.

The study, conducted during the period 1984 to 1985, was under the general supervision of Messrs. F. A. Herrmann, Jr., Chief of the Hydraulics Laboratory, WES; R. A. Sager, Assistant Chief of the Hydraulics Laboratory; Mr. M. B. Boyd, Chief of the Waterways Division, Hydraulics Laboratory; and under the direct supervision of Mr. W. A. Thomas, Research Hydraulic Engineer, Waterways Division, who was the Contracting Officer's Representative. This report was prepared by Mr. Madden as part of the contract, and was reviewed by Mr. Thomas.

At the time of publication of this report, Director of WES was Dr. Robert W. Whalin. Commander was COL Bruce K. Howard, EN.

Conversion Factors, Non-SI to SI Units of Measure

Non-SI units of measurement used in this report can be converted to SI units as follows:

Multiply	Ву	To Obtain
cubic feet	0.02831685	cubic meters
Fahrenheit degrees	5/9	Celsius degrees or kelvins ¹
feet	0.3048	metres
tons (2,000 pounds, mass)	907.1847	kilograms

¹ To obtain Celsius (C) temperature readings from Fahrenheit (F) readings, use the following formula: C = (5/9)(F - 32). To obtain Kelvin (K) readings, use: K = (5/9)(F - 32) + 273.15.

1 Introduction

During planning studies for the Arkansas River navigation channel, which were carried out during the late 1950's and on into 1960, it was considered desirable to express the relation between stream and channel characteristics, discharge, and bed-material sediment load in generalized terms such that the effects of changes in the various parameters involved could be evaluated. A functional relationship developed by Emmett M. Laursen (1968) was used as a framework for developing a generalized working curve for use in the Arkansas River channel design studies. Laursen's relationship was adopted because it is expressed in terms which permit separating readily the effects of the various parameters which are generally considered to govern the relation between the bed-material load, the hydraulic characteristics of the streamflow, and the characteristics of the material of which the streambed is composed. In addition, being empirical, the Laursen relation is susceptible of being adjusted to fit Arkansas River sediment load observations.

2 Laursen Procedure

Using the results of a number of flume tests from various sources Laursen developed a functional relation curve between the expressions $\sqrt{to/\rho/w}$ and $c/((d/D)^{7/6} ((to'/tc)-1))$, where $\sqrt{to/\rho}$ is the shear velocity at the streambed in feet per second, and the second group of parameters is referred to as $f(\sqrt{to/\rho/w})$; to is the boundary shear or tractive force in pounds per square foot, to' is the boundary shear associated with the sediment particles in the streambed, tc is the critical tractive force for beginning of movement of the sediment particles, ρ is the mass density of the fluid (1.94 for water), w is the fall velocity of the sediment particles in water in feet per second, c is the concentration of sediment in percent by weight, d is the diameter of the sediment particle (mean diameter of each fractional size range in feet, D is the depth of flow in feet, and f means "function of."

Laursen's functional relation curve is shown in Figure 1. In attempting to reproduce sediment load versus discharge rating curves which had been developed for gaging stations on the lower Arkansas River from numerous sediment measurements that had been made over a period of many years, it was discovered that the rating curves calculated from Laursen's relation resulted in loads considerably smaller than the curves developed from the long-term measurements. However, the curves did parallel each other. It was also noted that the data point values of $f(\sqrt{to/\rho/w})$ calculated from Missouri River data by D. C. Bondurant (1968) plotted considerably higher than Laursen's functional relation. For these reasons, a new relationship curve was developed for use in the Arkansas River planning studies, using Laursen's parameters but based on Arkansas River data. Two versions of the modified relationship were developed at different times. Both versions are shown on Figure 1 for comparison with Laursen's original curve.

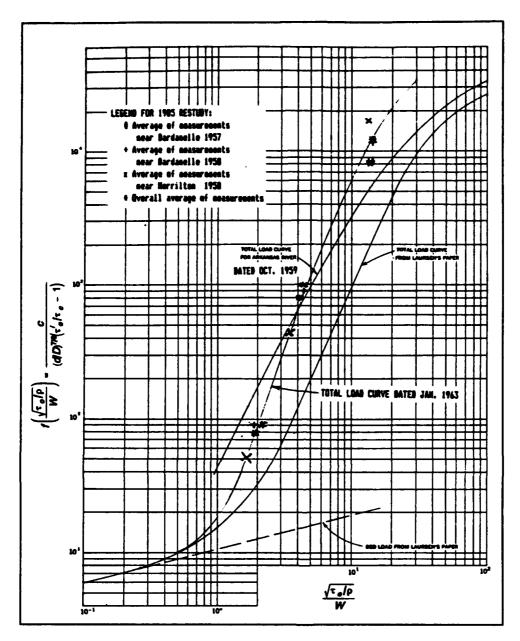


Figure 1. Relation for sediment load, Laursen method

3 Arkansas River Data

Three sets of special measurements were made on the Arkansas River as follows:

Near Dardanelle, Arkansas, in June-July 1957, Near Dardanelle, Arkansas, in April 1958, and Near Morrilton, Arkansas, in April 1958.

In each set, the measurements were made on four separate ranges and at five verticals on each range, resulting in 20 measuring locations in each set and a total of 60 locations for the three sets.

The observations at each vertical consisted of the sounded depth, the mean velocity in the vertical, and a depth-integrated suspended sediment sample. Bed-material samples were also obtained at each vertical with a revolving-bucket type sampler during the 1958 measurements at both Dardanelle and Morrilton. Attempts to obtain bed-material samples with a drag-bucket sampler at Dardanelle during the 1957 observations were unsuccessful. The water temperature was measured on each day of the observations. Water surface elevations also were obtained at each range. The total river discharges during the observations were approximately 178,000 cfs¹ at Dardanelle in 1957, 121,000 cfs at Dardanelle in 1958, and 97,000 cfs at Morrilton in 1958.

¹ A table of factors for converting non-SI units of measure to SI units is found on page v.

4 Development of Modified Laursen Functional Relationship

The sediment size classification used in this study is presented in the following tabulation:

Sediment-size Class	Size Range In mm	Geometric Mean for in mm	Size Class in feet
Coarse Silt	0.031 - 0.0625	0.044	0.000142
Very Fine Sand	0.0625 - 0.125	0.088	0.000285
Fine Sand	0.125 - 0.250	0.177	0.000580
Medium Sand	0.250 - 0.500	0.353	0.001158
Coarse Sand	0.500 - 1.00	0.707	0.00232
Very Coarse Sand	1.00 - 2.00	1.414	0.00464
Very Fine Gravel	2.00 - 4.00	2.828	0.00928

Sediment fall velocities as a function of grain size and water temperature are shown in Figure 2.

The procedure for developing the desired functional relationship consists essentially of calculating values of $\sqrt{\tau_o/\rho/w}$, $(d/D)^{7/6}$, and $((\tau'_o/\tau_c)-1)$ for each data point, based on the observed information on flow and bed-material characteristics and then solving for $f(\sqrt{\tau_o/\rho/w})$ using the equation:

$$f\left(\sqrt{\tau_o/\rho/w}\right) = (c/P_s)/\left\{P_b(d/D)^{7/6} \left[(\tau_o/\tau_c)-1\right]\right\}$$
 (1)

where P_s is the fraction of suspended material of the size class represented by d, P_b is the fraction of bed material of the size class represented by d, and the other symbols are as previously defined.

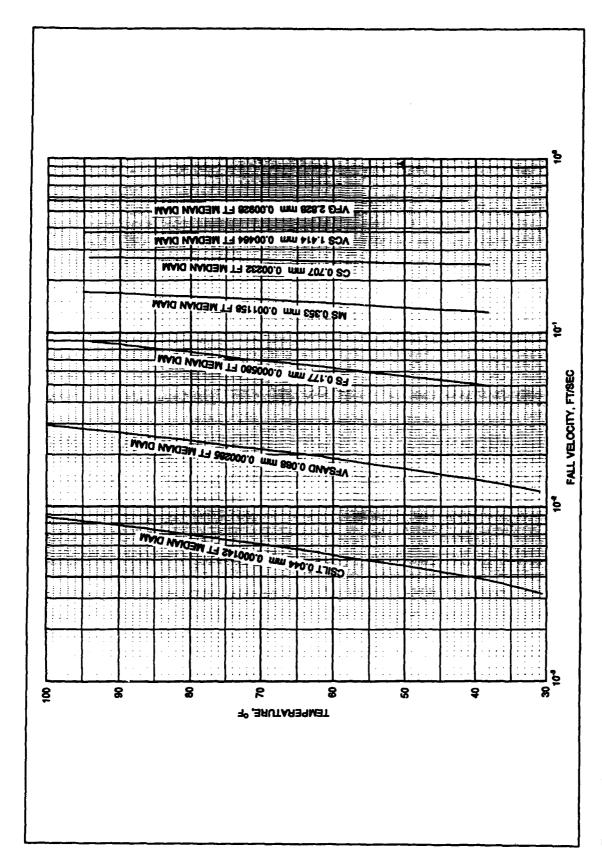


Figure 2. Sediment fall velocity

Additional pertinent equations are as follows:

$$\tau_o = \gamma DS = 28.25n^2 V^2 / D^{1/3} \tag{2}$$

$$\tau_o' = (V^2/30d_m/D)^{1/3} \tag{3}$$

$$\tau_c = 4d$$
 in general, (4a)

but
$$\tau_c > 4d$$
 for particles less than .088mm in size (4b)

$$q_s = 27qc \qquad or \ Q_s = 27Qc \tag{5}$$

In these equations γ is the specific weight of the fluid in pounds per cubic foot (62.4 for water), S is the energy gradient in feet per foot, n is the Manning roughness coefficient, V is the mean velocity in feet per second, d_m is the median size of the sediment mixture in the streambed in feet (considered representation of the grain roughness of the bed), q is the discharge per foot width in cubic feet per second per foot, Q is the total discharge in the stream cross section in cubic feet per second, q_s is the sediment load in tons per day per foot width, and Q_s is the total sediment load in the channel cross section in tons per day. These and other symbols are summarized in a list of symbols, Appendix A.

The procedure described above was applied to each sediment size class in the suspended and bed material samples at each observation location. These calculations resulted in values of $f(\sqrt{\tau_o/\rho/w})$ for the suspended sediment corresponding to each value of $(\sqrt{\tau_o/\rho/w})$. Values of $f(\sqrt{\tau_o/\rho/w})$ for bed load were calculated from the equation.

$$f(\sqrt{\tau_o/\rho}/w)_b = 10.7378 (\sqrt{\tau_o/\rho}/w)^{0.25301}$$
 (6)

which was deduced from Laursen's curve labelled "Bed load" in Figure 1. The bed-load values were added to the suspended-load values to obtain values of $f(\sqrt{\tau_o/\rho/w})$ applicable to the total load. Plotting of the resulting values of $f(\sqrt{\tau_o/\rho/w})$ versus corresponding values of $\sqrt{\tau_o/\rho/w}$ served as the basis for developing the functional relationship curve. As the many points were widely scattered, group averaging was employed to aid in plotting the curve. The points fell into groups according to sediment-size class. Accordingly, the group averaging was performed on a size-class basis.

The latest, 1985, implementation of the procedure described above is illustrated in detail by Table 1. In the interest of simplifying computer printouts, the symbols To, To', X, Y, and Y' have been substituted for τ_o , τ'_o , $\sqrt{\tau_o/\rho/w}$, $f(\sqrt{\tau_o/\rho/w})$ for suspended load, and $f(\sqrt{\tau_o/\rho/w})$ for total load, respectively. The results of computations for all of the special Arkansas River observations at Dardanelle and Morrilton in 1957 and 1958 are included in Appendix B of this report as Tables B-1 through B-12. The computation of group averages of data points is included as Table B-13.

At the time of the Arkansas River project planning studies, the results of laboratory analyses of the bed-sediment samples had not been completed. Because of this, it was necessary to use the results of bed-material samples obtained previously during relatively low river flows. The resulting modified functional relationship curve in Figure 1 is labelled "Curve dated October 1959." The application of that relationship curve to the Arkansas River project planning studies is described in Madden (1964).

The relationship curve was revised in 1963 utilizing the results of the bed-material samples that were obtained at the time of the special observations in 1958. The 1958 samples at Dardanelle were assumed to be applicable to the 1957 observations at Dardanelle in the absence of actual bed-samples at that time. The 1963 modified relationship curve is labelled "Curve dated Jan. 1963" in Figure 1. A more detailed "working-curve" version of the 1963 curve is included as Figure 3 of this report. Copies of this curve were distributed to attendees at a course in Sediment Problems in Hydraulic Engineering that was held at the US Army Engineer Waterways Experiment Station in Vicksburg, MS, May 18-22, 1970.

The group-averaged data points computed in the latest (1985) study agree very closely with the 1963 relationship curve. Consequently, further revision of that curve is considered not warranted.

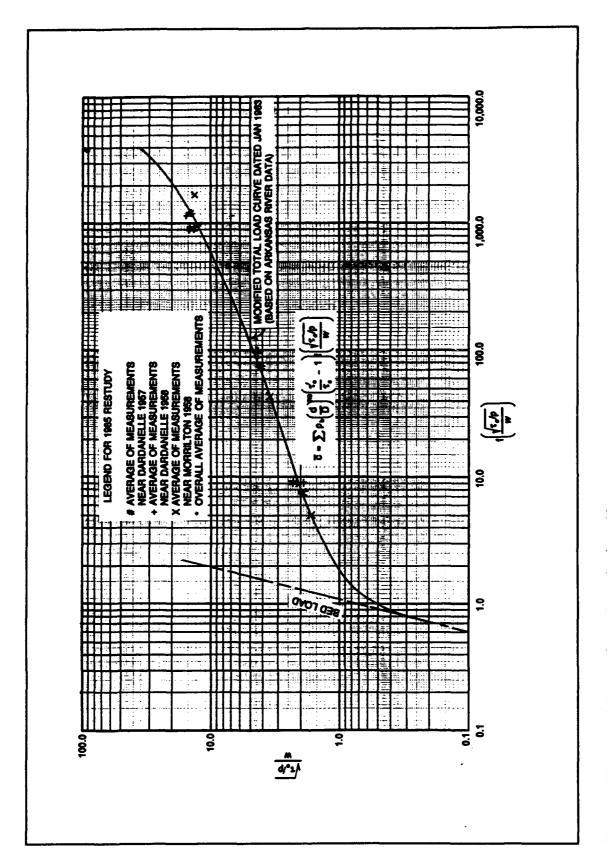


Figure 3. Modified relationship for sediment load, working curve

5 Application of Modified Laursen Functional Relationship

Calculation of the bed-material sediment concentration follows a reverse process to that described above. Data requirements include the flow depth or hydraulic radius, D or R; the velocity, V; the energy gradient, S; or a Manning n value; a grain-size distribution for the bed material, P_{D} ; and an observed or estimated water temperature, TDF. The parameters $\sqrt{\tau_O/\rho/w}$, (d/D)7/6, and $((\tau'_O/\tau_C)-1)$ are first computed from the known information as before. For each value of $\sqrt{\tau_O/\rho/w}$, a corresponding value of $f(\sqrt{\tau_O/\rho/w})$ is then read from the functional relationship curve. The sediment concentration is then calculated by means of the equation:

$$c = P_b(d/D)^{7/6} \quad ((\tau_o'/\tau_c) - 1)f(\sqrt{\tau_o/\rho} / w) \tag{7}$$

The sediment load is calculated from Equation 5.

The calculations are carried out for each grain-size class, and the resulting incremental loads are then summed to obtain the combined load for all sizes. For total load, Equation 7 is modified as follows:

$$\bar{C} = \sum P_b (d/D)^{7/6} ((\tau_o'/\tau_c) - 1) f(\sqrt{\tau_o/\rho}/w)$$
 (7a)

where \overline{C} is the total bed-material concentration and \sum represents summation.

As a test of the procedure, it has been applied to the following locations at which observed sediment concentration data are available for comparison with computed values:

RIVERS:

Atchafalaya River at Simmesport, Louisiana Mississippi River at Tarbert Landing, Louisiana Mississippi River at St. Louis, Missouri
Red River at Alexandria, Louisiana
Rio Grande near Bernalillo, New Mexico
Middle Loup River at Dunning, Nebraska
Niobrara River near Cody, Nebraska
Arkansas River at Dardanelle and Morrilton, Arkansas

FLUME TESTS:

Simons and Richardson, 0.19mm sand, Colorado State University

" " 0.27mm sand " " " 0.45mm sand " " " " 0.45mm sand " " " " " 0.93mm sand " " " " Toch, O.04mm sand, Iowa Institute of Hydraulic Research

The information on all of the rivers except the Arkansas was obtained from a paper by Toffaleti (1968). Information on the flume tests by Simons and Richardson was obtained from Guy, Simons, and Richardson (1966). Information on the flume tests by Toch was obtained from Laursen (1968).

Calculation of the bed-material sediment load is demonstrated in detail in Table 2. Calculations for all of the locations listed above are included in Appendix C as Tables Nos. C-1 through C-27. Critical tractive force values of 4d were assumed for all computations except for the Toch flume tests, for which a value of 5d was used because of the small size of the bed material. The tables include computations of ratios of computed load to observed load. A wide variation in the ratios can be noted. In an investigation to determine whether or not some additional parameters should be included in the procedure, the ratios were plotted against values of the Froude number. $F_r = V/\sqrt{gD}$, where g is the gravitational acceleration. The Froude number is considered to be one of the factors governing the presence of ripples, dunes, antidunes, plane bed, or intermediate transitions. (See Albertson, Simons, and Richardson (1958) and a relationship of dune wave steepness versus Froude number presented in Vanoni (1975) from a study by Kennedy (1963).) Variations in these bed-regime features affect the bed roughness and turbulence. which, in turn, affect the flow-sediment interaction.

The plot of the computed-to-observed load ratios versus Froude numbers on log-log graph paper is shown in Figure 4. A definite correlation can be observed. A representative straight-line curve has been drawn in an approximately median position among the points. Most of the points lie within enveloping curves drawn at positions giving ratio values from one-half to two times the median curve values. This degree of correlation is considered good for field sediment data. Almost all of the points are within a range of one-third to 3 times the median values. This is considered acceptable.

Two of the points diverge widely from the others. An examination of the basic information on these points revealed that sediment transport was very small, consisting entirely or almost entirely of bed-load movement with little

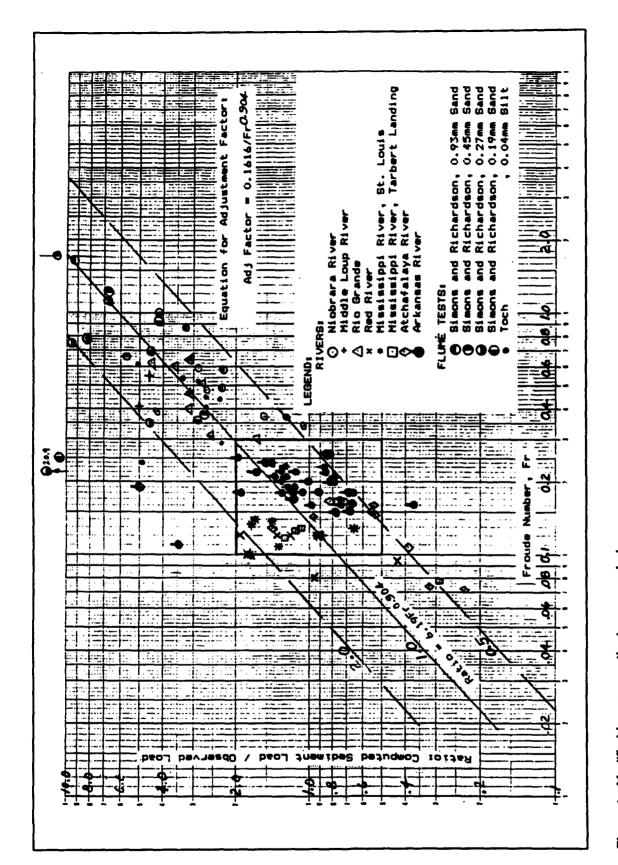


Figure 4. Modified Laursen method, error analysis

or no suspended load. The bed configuration was in the ripple regime. Also, it was noted that the grain-associated tractive force, τ_o , for the median-size was only 1.4 and 1.5 times the computed critical tractive force, τ_c . This suggests the possibility of a "hiding effect," in which the smaller particles are partially sheltered when movement of the median size is marginal, or that the assumed value of 4d does not define the critical tractive force with sufficient accuracy under near-threshold conditions.

The following equation was deduced for the median curve of relationship between the ratio of computed to observed sediment load and the Froude number:

$$Ratio = 6.19F_r^{0.904}$$
 (8)

An adjustment factor for adjustment of the computed load can be computed from the inverse of the latter equation:

$$Adj. \ Factor = 0.1616/F_r^{0.904}$$
 (9)

Equation 9 was applied to each initially computed load or concentration in Tables 2 and B-1 through B-27 to obtain adjusted values of computed load or concentration. Although the adjustment was performed as a separate operation in the tables for illustrative purposes, it should be noted that Equation 9 can be incorporated into Equations 7 and 7a, resulting in the equations:

$$c = P_b(d/D)^{7/6} \left((\tau_o'/\tau_c) - 1 \right) f(\sqrt{\tau_o/\rho} / w) (0.1616/F_r^{0.904})$$
 (10)

and
$$\bar{C} = \sum P_b(d/D)^{7/6} ((\tau_o'/\tau_c) - 1) f(\sqrt{\tau_o/\rho} / w) (0.1616/F_r^{0.904})$$
 (10a)

As indicated previously, the bed-material load is computed by means of the equation:

$$q_s = 27q\overline{C}$$
 or $Q_s = 27Q\overline{C}$ (11)

A plot of all values of adjusted computed loads or concentrations versus observed loads or concentrations, shown in Figure 5, indicates acceptable results, comparable to results of other sediment load computation procedures.

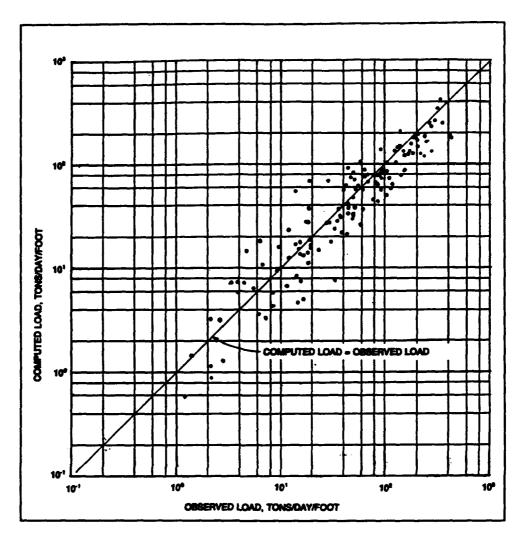


Figure 5. Comparison of results

6 Range of Applicability

The modified Laursen procedure has been applied to sediments ranging in size from coarse silt (noncohesive) to very fine gravel, flow depths ranging from 0.25 to 54 feet, velocities from 0.85 to 7.7 feet per second, energy gradients from 0.00001 to 0.1, temperatures from 36 to 90 degrees Fahrenheit, and Froude numbers from 0.07 to 1.7. It is concluded that the results, with adjustments for Froude number effects, are satisfactory throughout these ranges in variables except when the grain-associated tractive force for the median size of the bed-material mixture is less than about two times the critical tractive force. Within this same restriction, satisfactory results can be obtained without the Froude number adjustment when the Froude number is within the range from 0.10 to 0.30 (see boxed area in Figure 4.) This range of Froude numbers is characteristic of large alluvial rivers.

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Appendix A List of Symbols

- c sediment concentration of each grain size class, percent by weight \bar{C} total sediment concentration of all grain size classes, percent by
- weight
- diameter of sediment particle (geometric mean of size class; $d = \sqrt{d_i * d_{i+1}} \quad \text{where } i \text{ represents the lower bound and } i+1$ the upper bound of the size class), ft
- d_m median size of bed material, ft (i.e., D_{50})
- D depth of flow in a vertical, ft
- f() function of variable inclosed in the parentheses
 - F_r Froude Number, V/\sqrt{gD}
 - g gravitational acceleration, ft/sec/sec
 - n roughness coefficient in Manning flow formula
 - P_b fraction of bed material of diameter d, % by weight
 - P_s fraction of suspended material of diameter d
 - q flow per unit width, VD or Q/W, cfs/ft
 - q_s sediment load per unit width, tons/day/ft
 - Q total rate of flow in a cross section, cfs
 - Q_s total bed material sediment discharge, tons/day

- R hydraulic radius of a channel cross section, ft
- S energy gradient, ft/ft
- Tc substitute symbol for τ_c, critical tractive force for beginning of sediment movement, lb/sq ft
- To' substitute symbol for τ_c , boundary shear or tractive force associated with sediment particles, lb/sq ft
- TDF temperature of water, degrees Fahrenheit
 - V velocity of flow, ft/sec
 - w fall velocity of sediment particle of size (or size class) d, ft/sec
 - W surface width of channel cross section, ft
 - X equivalent to $\sqrt{\tau_0/\rho/w}$ (also = $\sqrt{gDS/w}$ or $\sqrt{gRS/w}$), dimensionless
 - Y function of X or f(X) for suspended sediment concentration
 - Y' f(X) for total concentration including bed load
 - y specific weight of water, lb/cu ft
 - ρ mass density of fluid, 1.94 for water, slugs/cu ft
 - τ_o boundary shear or tractive force (= γDS), lb/sq ft
- $\sqrt{\tau_o/\rho}$ boundary shear velocity U_* (also = \sqrt{gDS}), ft/sec
 - ∑ sum

Appendix B Development of Modified Laursen Sediment Relationship Based on Arkansas River Data

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Appendix C Modified Laursen Method Sediment Load Calculations

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6. AUTHOR(S) Edward B. Madden			
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13. ABSTRACT (Maximum 200 words) The sediment transport function tion project because it expresses tra ment parameters. However, in attention gave results that were system Missouri River data. Therefore, La	insport rate using terms that mpting to reproduce measurematically low. The same	at permit separating the ured data from the low trend appeared when	ne effects of hydraulic and sedi- wer Arkansas River, the Laursen

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was replotted for the Arkansas River planning studies based on Arkansas River data. Subsequently, another graph of the relationship was developed using data from several other rivers. The work reported in this study is an effort (Continued)

14. SUBJECT TERMS Bed-material load Froude number	Sediment transport Streambed	15. NUMBER OF PAGES 69
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to collapse those functional relationships into a single graph. The approach was to introduce a correction coefficient based on Froude number.

The resulting relationship was tested using data from eight field sites and five flume studies. Results, with adjustments for Froude number effects, are satisfactory for sediment sizes ranging from 0.031 mm to 4 mm, flow depths from 0.25 to 54 ft, flow velocities from 0.85 to 7.7 fps, energy gradients from 0.00001 to 0.1 ft/ft, water temperatures from 36 to 90° F, and Froude numbers from 0.07 to 1.7 except when the grain tractive force is less than about two times the critical tractive force. Sediment transport is very small in this case, and there is probably a hiding effect beyond that included in Laursen's formulation.